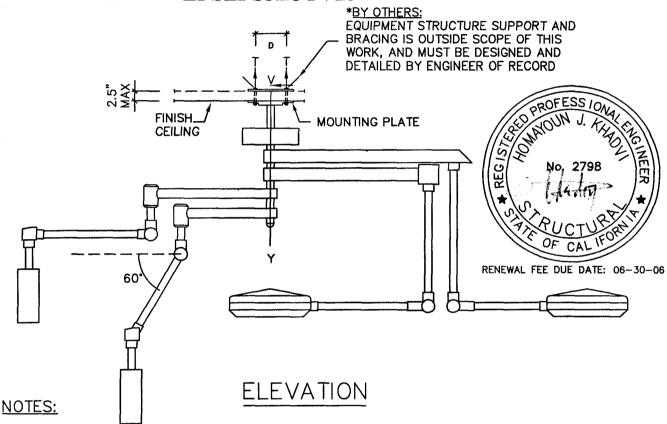
FIROUZI CONSULTING ENGINEER, INC.					
SKYTRON SURGICAL PRODUCTS	DES.	SHEET			
LFSLFSst23TV23	FCE JOB No.	1			
FOR SEISMIC ZONE (4), SOIL PROFILE (Sd) NEAR SOURCE FACTOR = 1.5	DATE: 4-6-04	OF 4 SHEETS			

SEISMIC ANCHORING BOLT DESIGN

LFSLFSst23TV23



- 1. SCOPE OF WORK: DESIGN OF BOLTS CONNECTING MOUNTING PLATE TO STRUCTURE ONLY.
- 2. FORCES ARE DETERMINED PER 2001 CALIFORNIA BUILDING CODE SECTION 1632A, (INCLUDING UP TO DATE REVISIONS) AND HAVE BEEN FACTORED TO REPRESENT WORKING DESIGN LOADS, NOT ULTIMATE
- 3. FORCES ARE MAXIMUMS AND OCCUR WHEN EQUIPMENT IS MOVED TO ITS MOST ECCENTRIC POSITION.
- 4. PROVIDE CEILING STRUCTURE DESIGNED AND DETAILS TO SUPPORT WEIGHTS AND FORCES SHOWN (BY ENGINEER OF RECORD FOR THE BUILDING)
- 5. ENGINEER OF RECORD TO DESIGN, DETAIL AND VERIFY STRUCTURE AND/ OR EXISTING LIGHT SUPPORT TRACTS TO SUPPORT INDICATED LOADS
- 6. HORIZONTAL FORCES AND MOMENT MAY OCCUR IN ANY DIRECTION, ACTING AT THE TOP OF MOUNTING PLATE.

FIROUZI CONSULTING ENGINEER, INC.				
SKYTRON SURGICAL PRODUCTS	DES.	SHEET		
LFSLFSst23TV23 FOR SEISMIC ZONE (4), SOIL PROFILE (Sd)	FCE JOB No.	2		
NEAR SOURCE FACTOR = 1.5	DATE: 4-6-04	OF 4 SHEETS		

DESIGN CRITERIA:

FORMULA 32A-1: $F_P = 4.0 \text{ Ca*Ip*Wp}$

TABLE 16A-Q : Ca = 0.44*Na = 0.44*1.5 = 0.66 (For zone 4 & SD)

TABLE 16A-K : Ia = 1.5 (For essential facility)

 \therefore F_P = (4.0)(0.66)(1.5)Wp = 3.96 Wp (For LRFD)

 $F_P = 3.96Wp/1.4 = 2.83Wp$ (For ASD)

FORMULA 30A-1: $E = p*E_h + E_v$

 $E_h = F_P$

p = 1.0 (FOR COMPONENT)

 $E_v = (0.5)Ca*Ip*Wp$

= (0.5)(0.66)(1.5)Wp = 0.5Wp (For LRFD)

= 0(For ASD)

SECTION 1630A.11: $E_v = (0.7)Ca*I*Wp$ = (0.7)(0.66)(1.5)/1.4 = 0.5Wp (For ASD) [NET UPLIFT FORCE]

LOAD COMBINATION CASE A

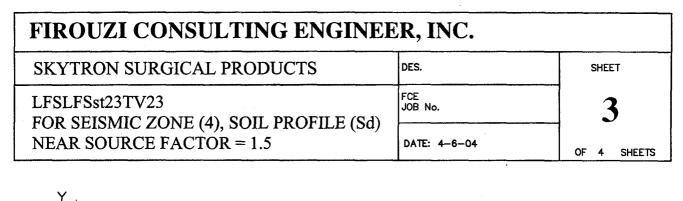
 $E_h = F_p$ DI + FV

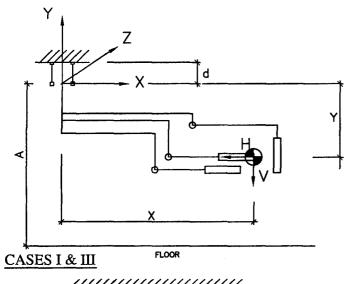
LOAD COMBINATION CASE B

$$E_V = 0.5DL$$

$$E_h = F_D$$

BY COMPARISION LOAD, COMBINATION A GOVERNS





FLOOR

CASES II & IV

Momment/bolt: m

Shear/bolt

t: Direct tension/bolt

t': Flexual tension/bolt

DISTANCE A = 10'-0" = 120" Vd : Dead Load (= DL)

A: MAX FLOOR TO MOUNTING PLATE

Ve : Vertical Seismic Load (= Ev)

He: Horizontal Seismic Load (= Eh)

CASE I (FIXTURE AT HIGH POSITION)

d = 2.5 " D = 9.5 "

Vd = 135.0 + 135.0 + 110.0 + 100.0 = 480.0 # Y = 120.0 - 80.0 = 40.0 "

 $X = (135.0 \times 90.1 + 135.0 \times 82.5 + 110.0 \times 70.5)$

 $+ 100.0 \times 62.8$)/ 480 = 77.8 "

 $Ve = 0.50 \times 480.0 = 237.6 \#$ $Ve = 0.50 \times 480.0 = 1357.7 \#$

He = 2.83 x 480.0 = 1357.7 # S = 1357.7 / 4.0 = 339.4 #

t = (480.0 + 237.6)/4 = 179 #Total Mzz = (480.0 + 237.6)x 77.8 + 1357.7 x40.0 = 110126 "#

t' = 110126 /(9.5 x 2) = 5796 # m = 339 x 2.5 = 848.6 "#

CASE II (FIXTURE AT LOW POSITION)

d = 2.5 " D = 9.5 "

Vd = 480.0 # Y = [(120.0 - 34.6)x 135.0 +(120.0 - 34.6)x 135.0 +(40.0 + 35.3)x 110.0 +(40.0 + 35.3)x

100.0]/ 480.0 = 81.0 " $X = [(50.6 + 39.5 \times \text{COS}(60)) \times 135.0 + (43.0)]$

+ 39.5 x COS(60))x 135.0 + 35.3 x 110.0 + 27.5 x 100.0]/ 480.0 = 51.3 "

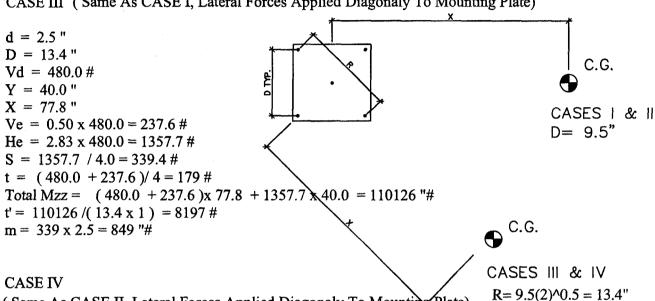
Ve = 0.50 x 480.0 = 237.6 # He = 2.83 x 480.0 = 1357.7 #

S = 1357.7 / 4.0 = 339.4 # t = (480.0 + 237.6)/4 = 179 # Total Mzz = (480.0 + 237.6)x 51.3 + 1357.7 x

81.0 = 146729 "# t' = 146729 /(9.5 x 2) = 7723 # m = 339 x 2.5 = 849 "#

F	TROUZI CONSULTING ENGINER	ER, INC.		***************************************	
S	KYTRON SURGICAL PRODUCTS	DES.	SHEET 4		
	FSLFSst23TV23	FCE JOB No.			
	FOR SEISMIC ZONE (4), SOIL PROFILE (Sd) NEAR SOURCE FACTOR = 1.5	DATE: 4-6-04	OF	4	SHEETS

CASE III (Same As CASE I, Lateral Forces Applied Diagonaly To Mounting Plate)



(Same As CASE II, Lateral Forces Applied Diagonaly To Mounting Plate) d = 2.5"

D = 13.4"

Vd = 480.0 #

Y = 81.0"

X = 51.3"

 $Ve = 0.50 \times 480.0 = 237.6 \#$

He = $2.83 \times 480.0 = 1357.7 \#$

S = 1357.7 / 4.0 = 339.4 #

t = (480.0 + 237.6)/4 = 179 #Total Mzz = (480.0 + 237.6)x 51.3 + 1357.7 x 81.0 = 146729 "#

 $t' = 146729 / (13.4 \times 1) = 10921 \# GOVERNS$

 $m = 339 \times 2.5 = 849$ "#

CHECK 7/8" DIA. A490 BOLTS:

ALLOWABLE TENSION: 32500 # ALLOWABLE SHEAR: 12600#

 $S = 3.14*d^3/32 = 3.14 \times (0.88)^3/32.0 = 0.07$ "3

fb = 848.6 / 0.07 = 12909 PSI

 $Fb = 0.75 \times 36000 = 27000 PSI$

fv/Fv + ft/Ft + fb/Fb =0.03 + 0.34 + 0.48 = 0.84 < 1.0 OK

USE 7/8" DIA. A490 BOLTS